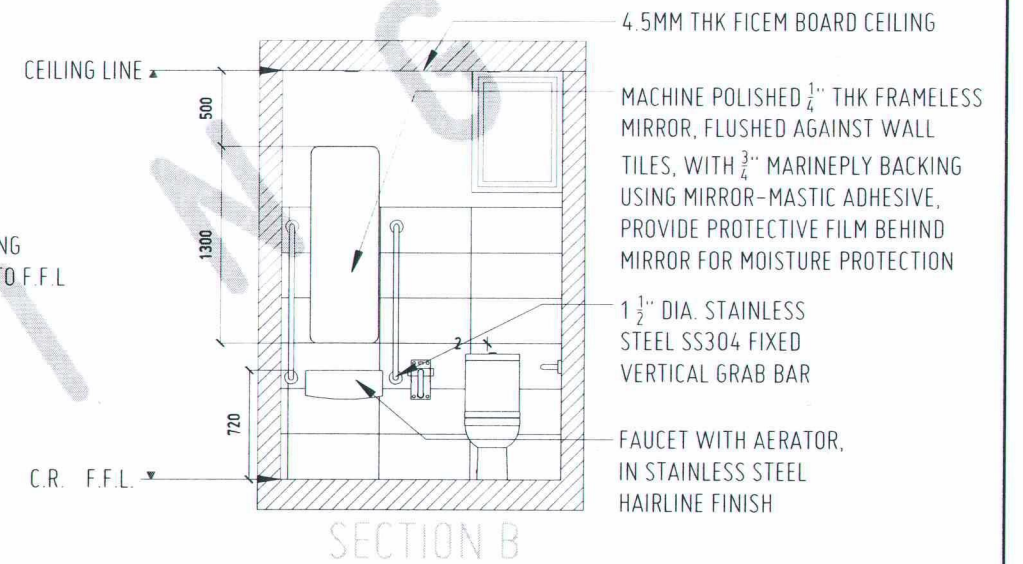
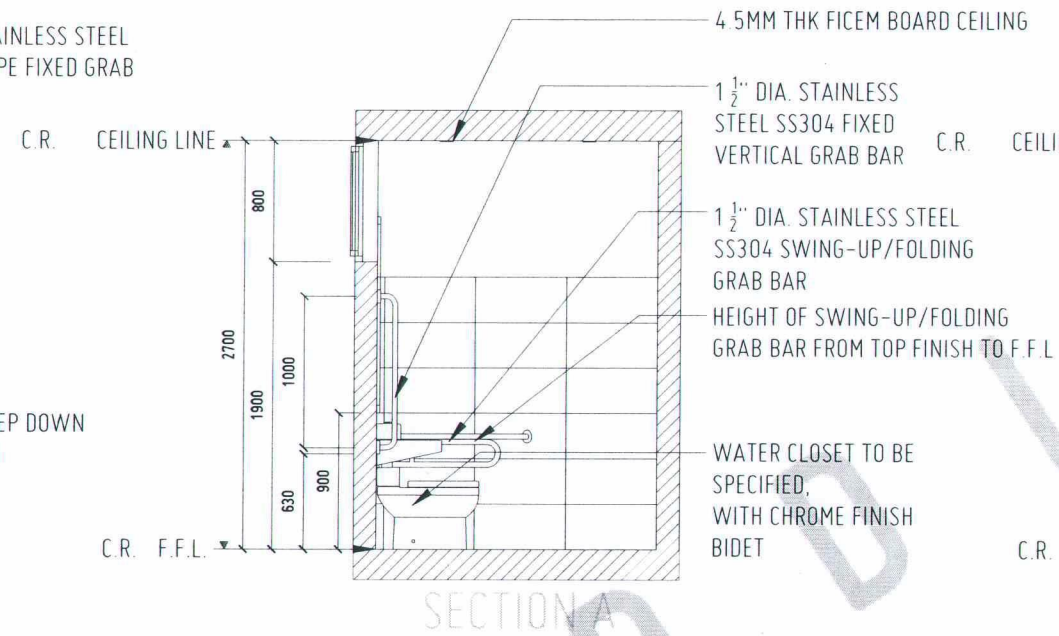
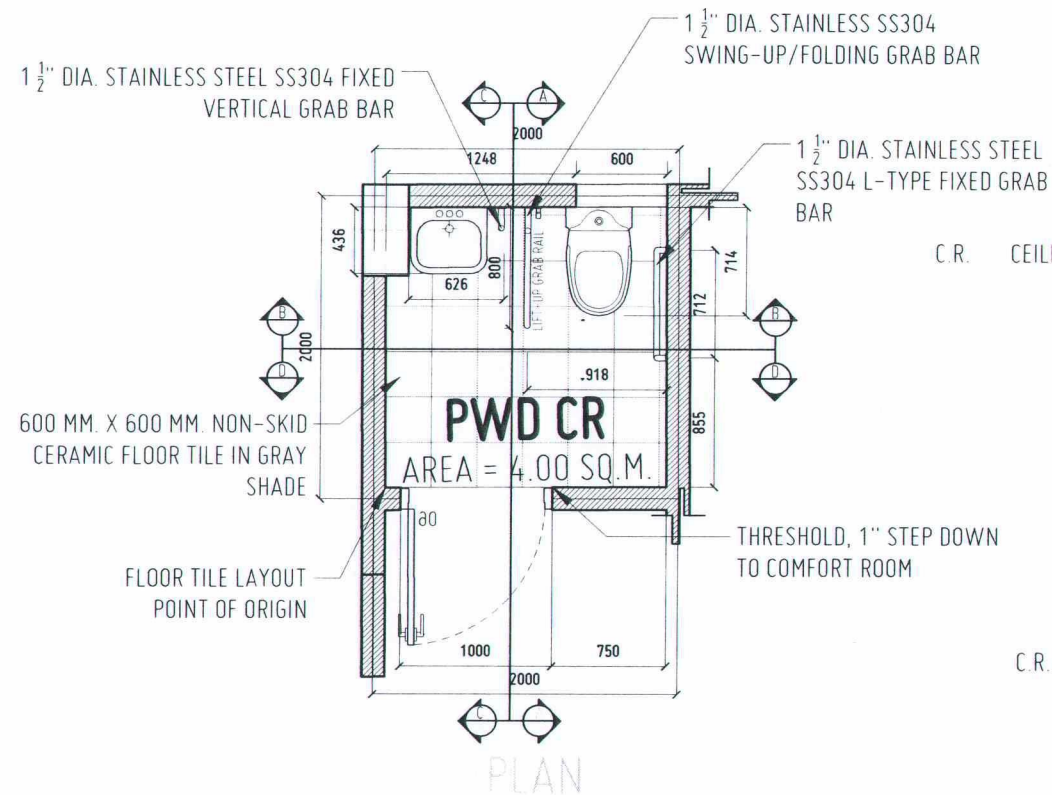
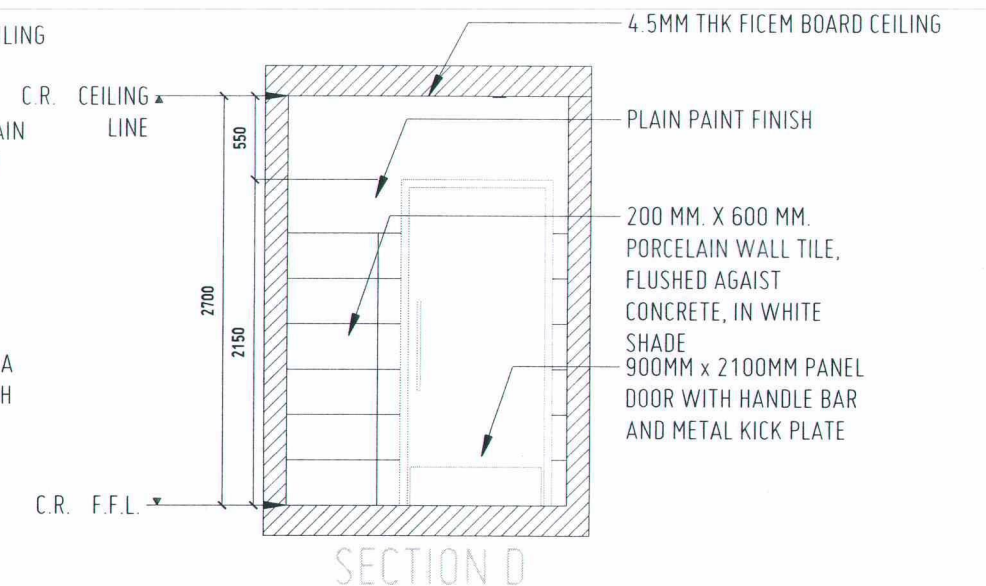
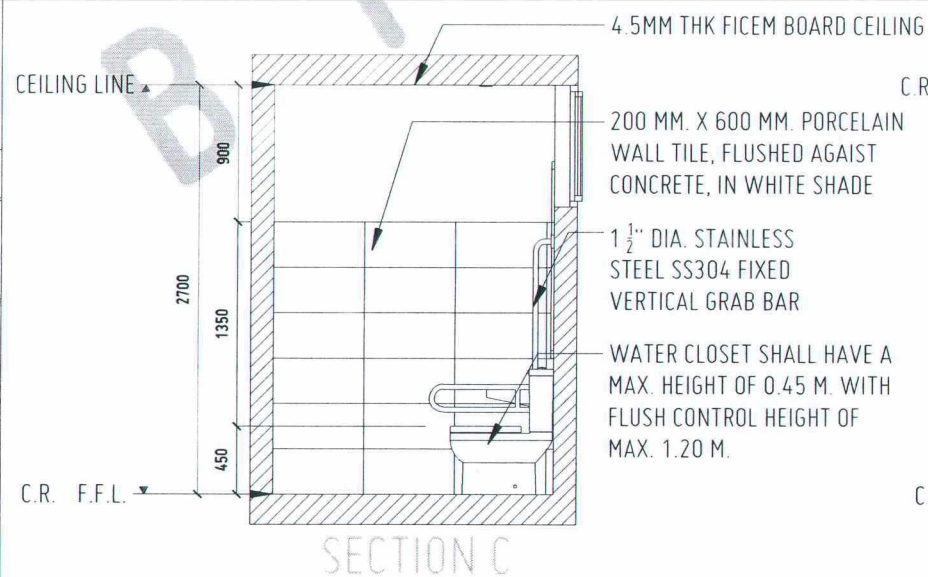
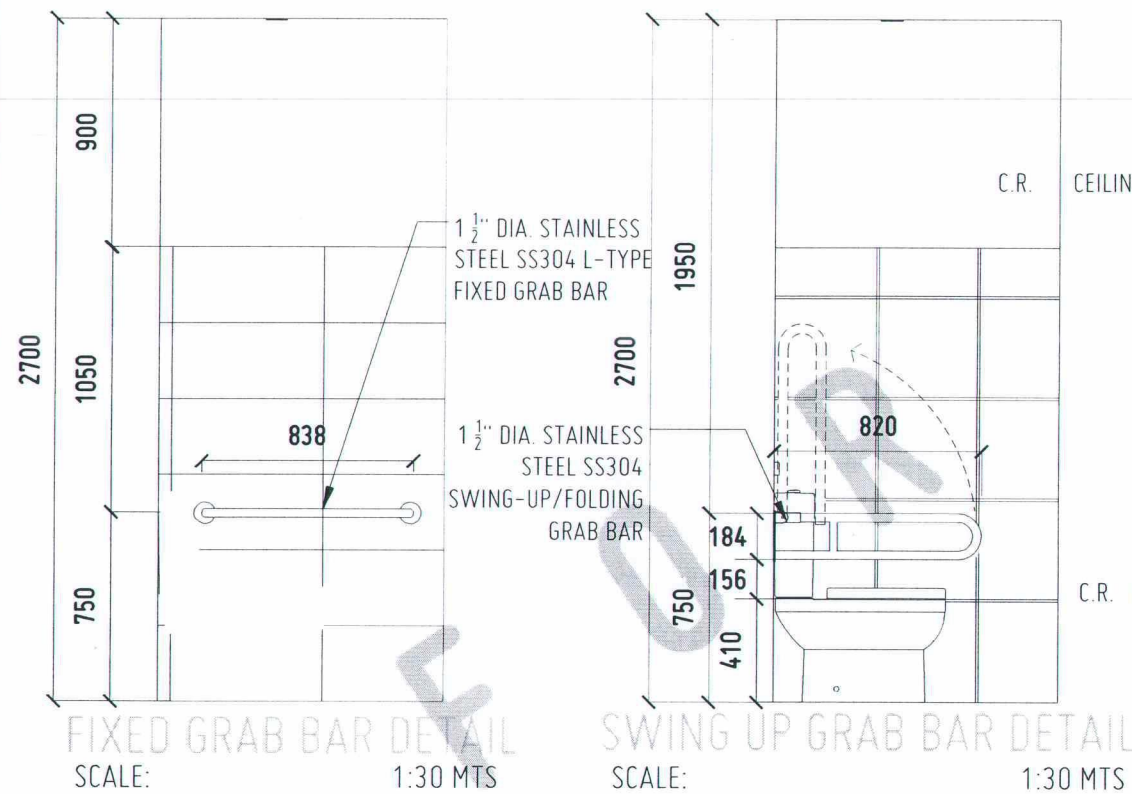




stainless steel
SS304 bidet spray

water closet, to
be specified

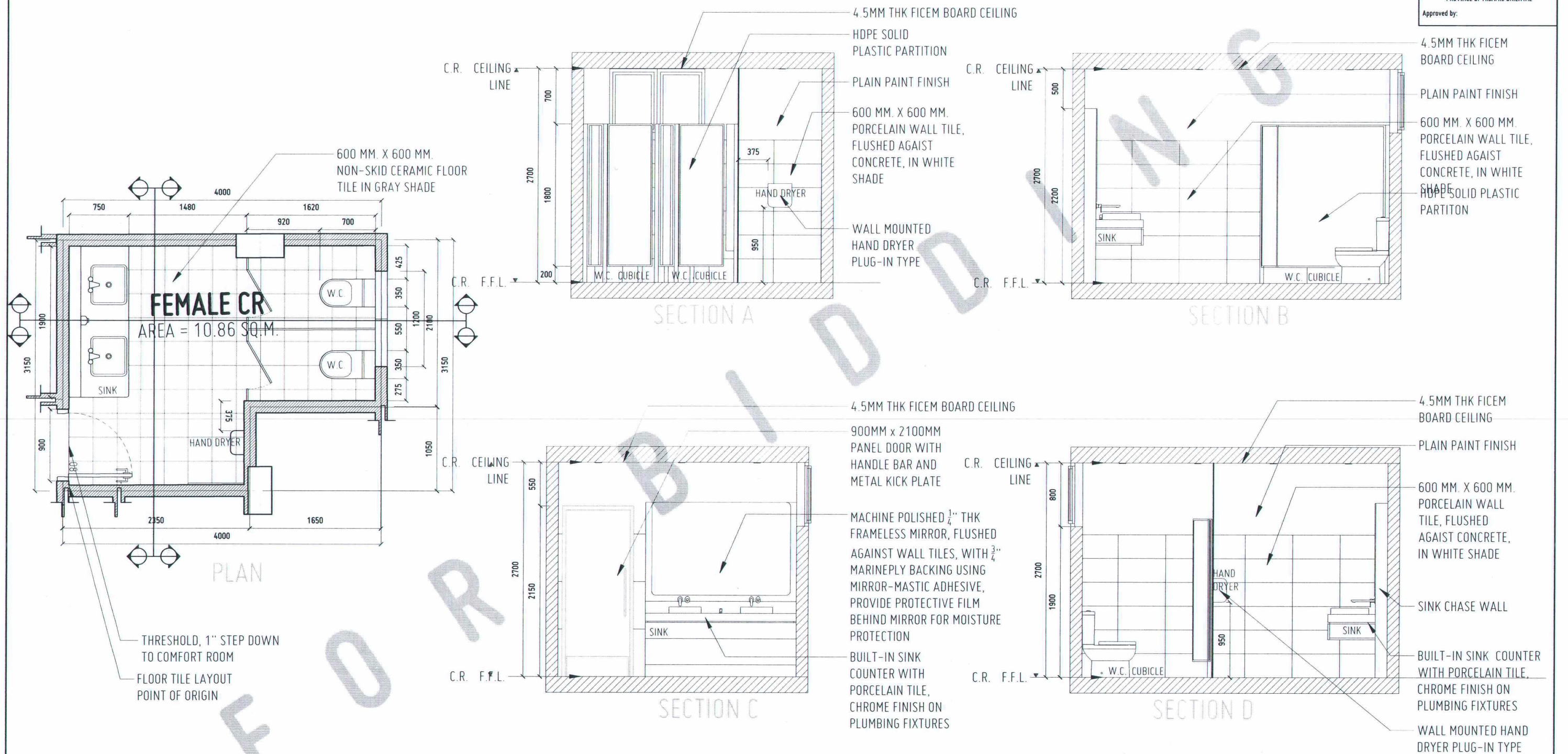


TYPICAL PWD COMFORT ROOM DETAIL

SCALE: 1:50 MTS

A
39
1

Approved by:



A
40
1

TYPICAL FEMALE COMFORT ROOM DETAIL

SCALE:

1:50MTS



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AR. FERDINAND A. DUMPA
ARCHITECT
PROJECT NO. 0913228-PRJ NO. 0589271 A
DATE 07-10-2026
TIN NO. 185-092-837 PLACE CDO

CONSTRUCTION OF
INNOVATION AND COLLABORATION HUB PHASE II,
ALUBUIJOD (MAIN) CAMPUS
LOCATION USTP AGILA S&T PARK, ALUBUIJOD, MISAMIS ORIENTAL
OWNER UNIVERSITY OF SCIENCE & TECHNOLOGY OF SOUTHERN PHILIPPINES

RECOMMENDING APPROVAL:
ENGR. GRACE C. BABA
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FACILITY DEVELOPMENT OFFICE

RECOMMENDING APPROVAL:
DR. LORY LIZA D. BULAY-OG
DIRECTOR, USTP ALUBUIJOD

RECOMMENDING APPROVAL:
ATTY. JONATHAN S. OCHE
VP FOR ADMINISTRATION & LEGAL AFFAIRS

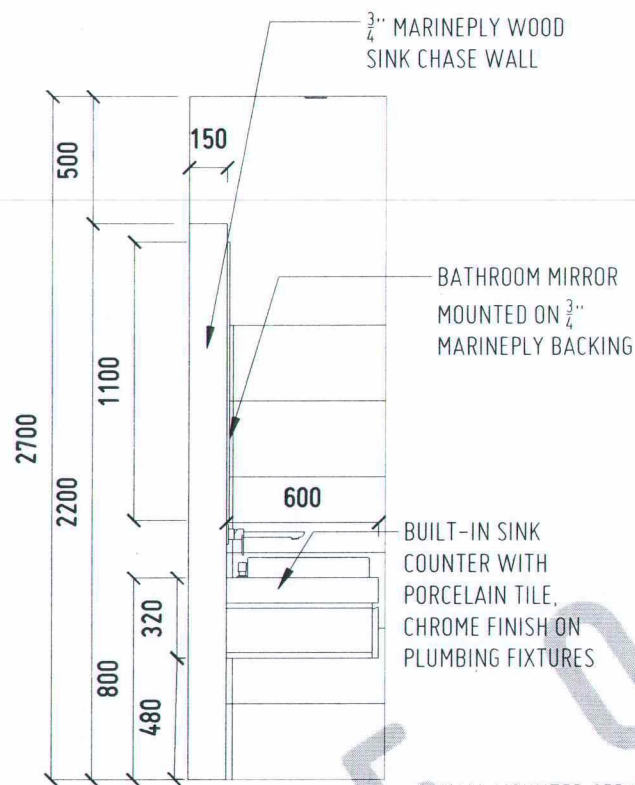
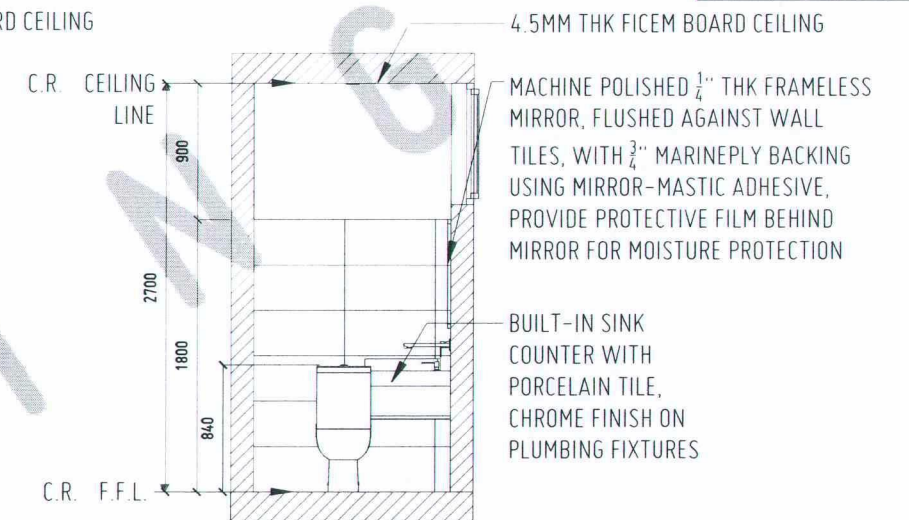
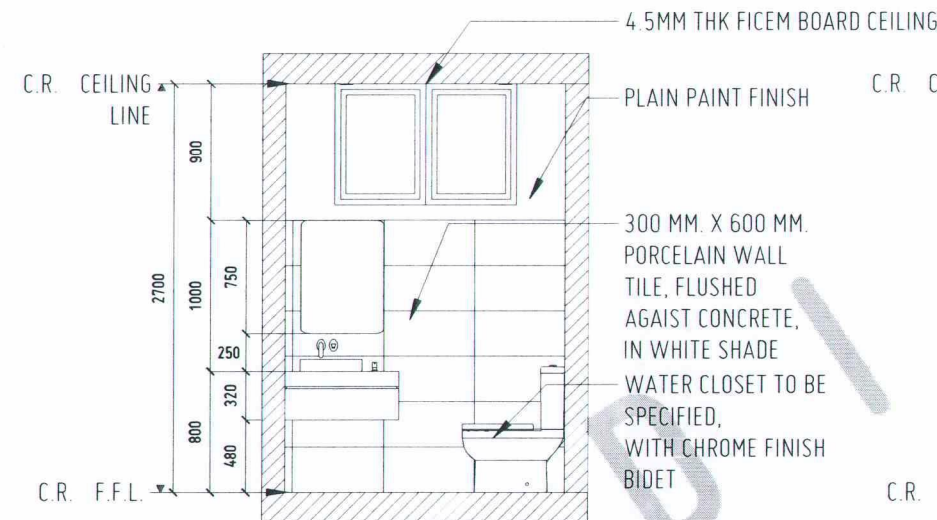
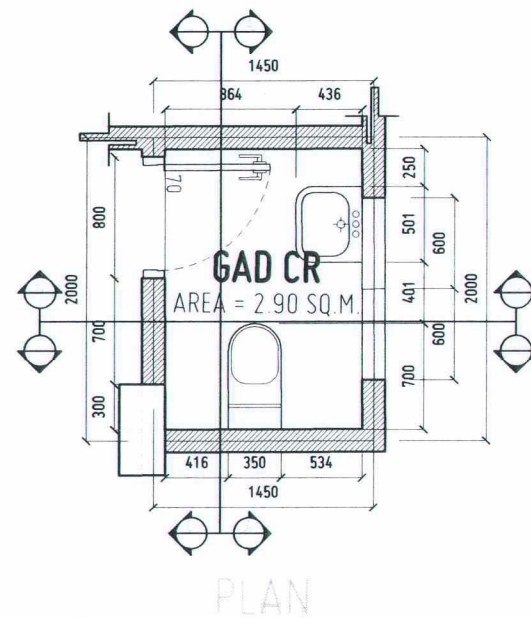
APPROVED BY:
DR. AMBROSIO S. CULTURA II
PRESIDENT, USTP SYSTEM

SHEET CONTENTS:

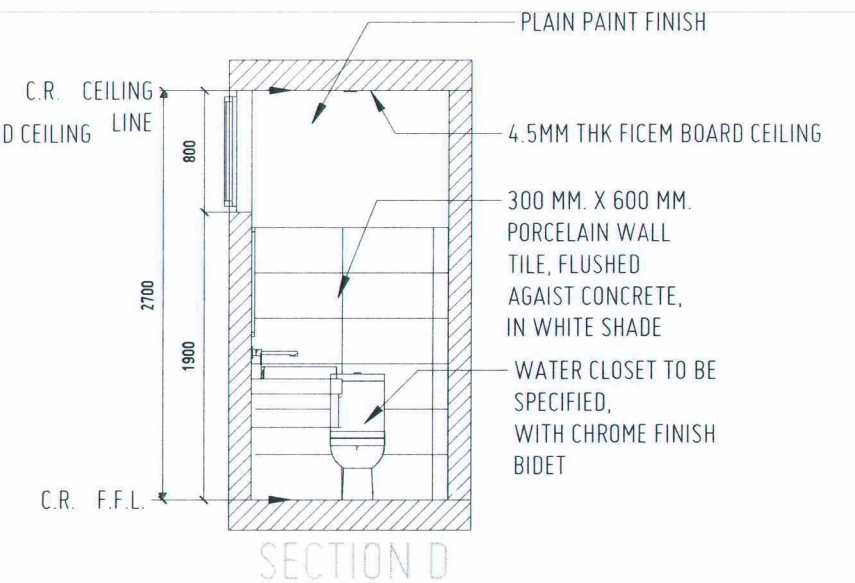
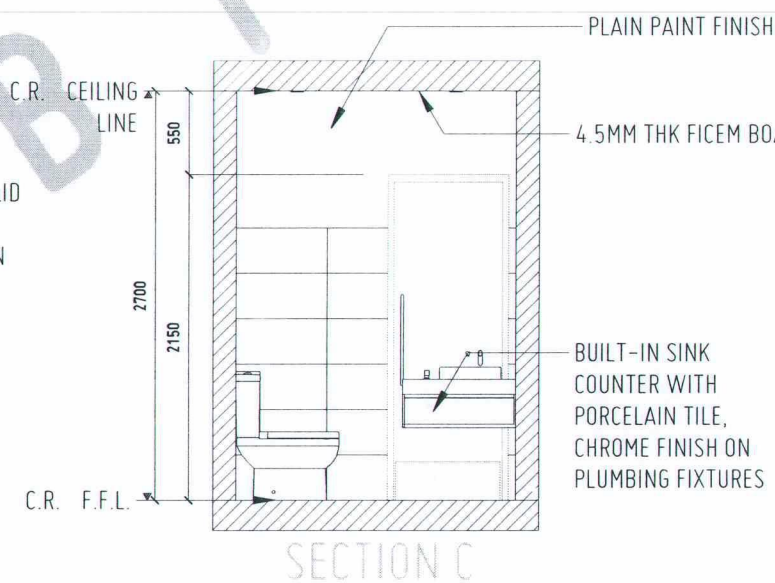
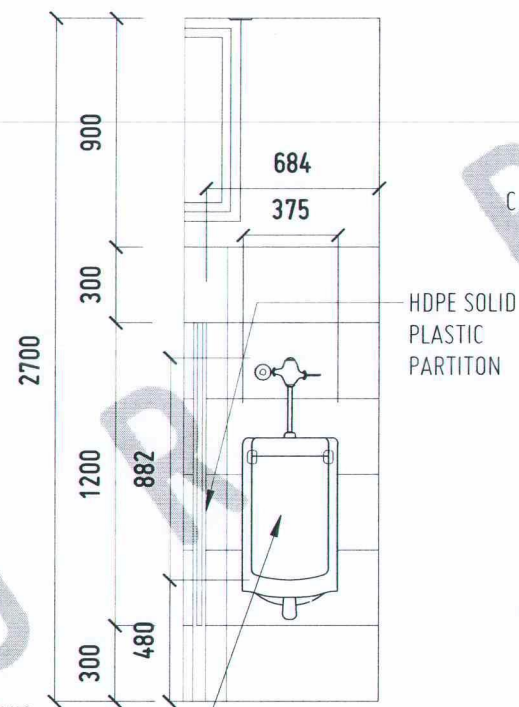
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DATE DRAWN:
FNT:

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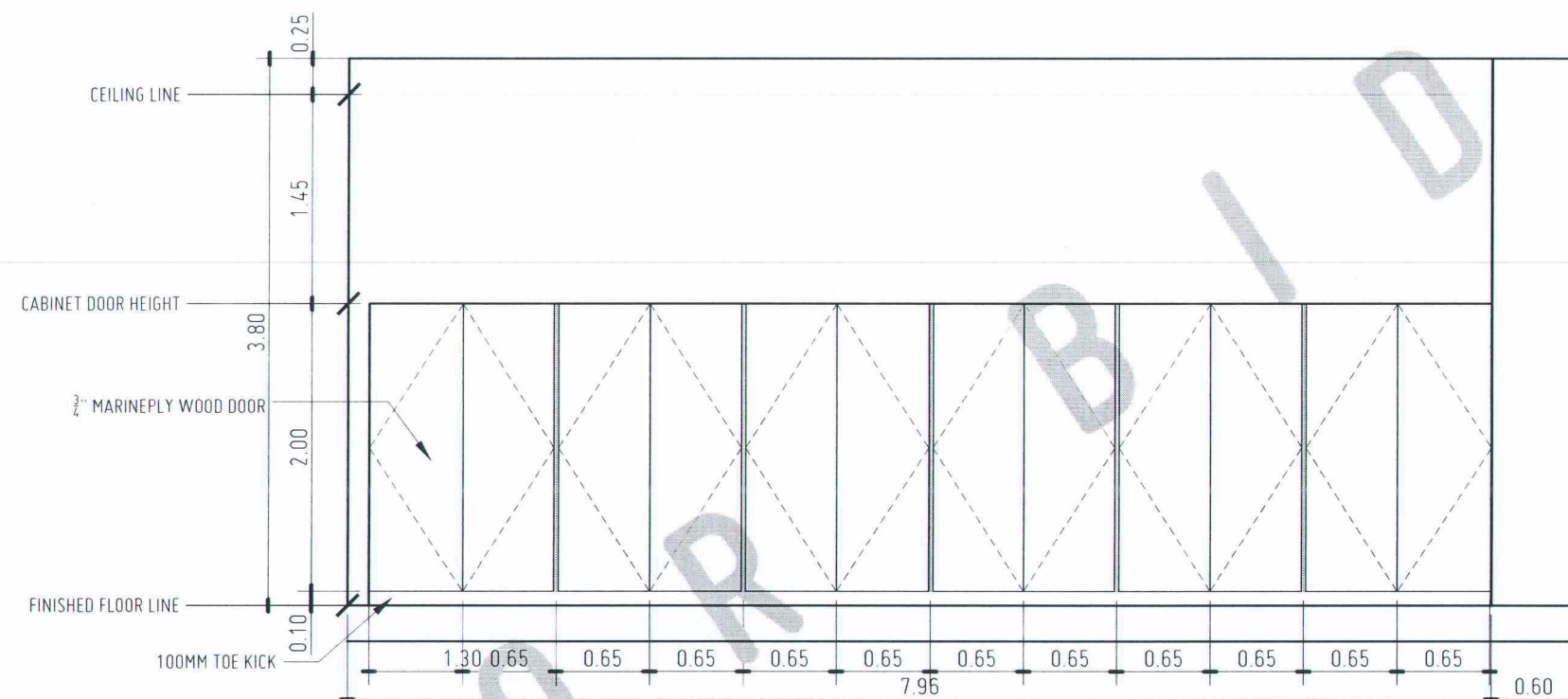
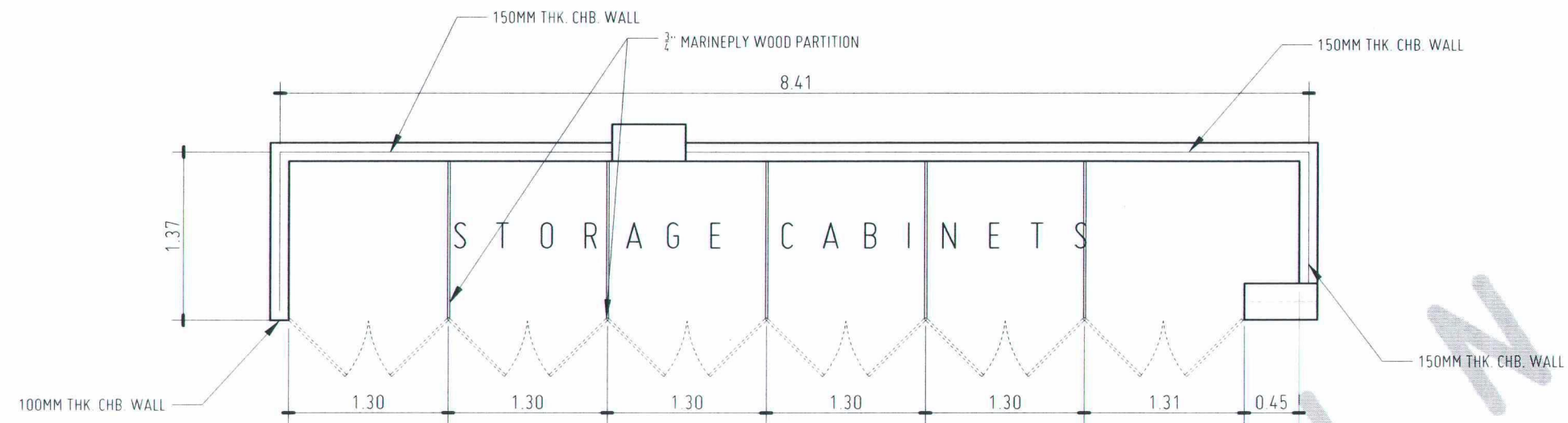
WALL MOUNTED CERAMIC URINAL WITH ELONGATED LIP OR TROUGH TYPE



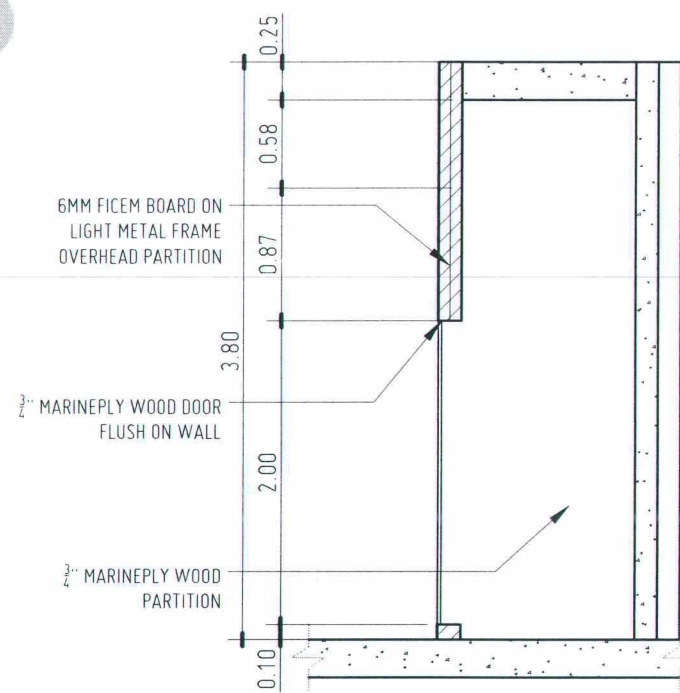
TYPICAL GAD COMFORT ROOM DETAIL

SCALE: 1:50 MTS





ELEVATION



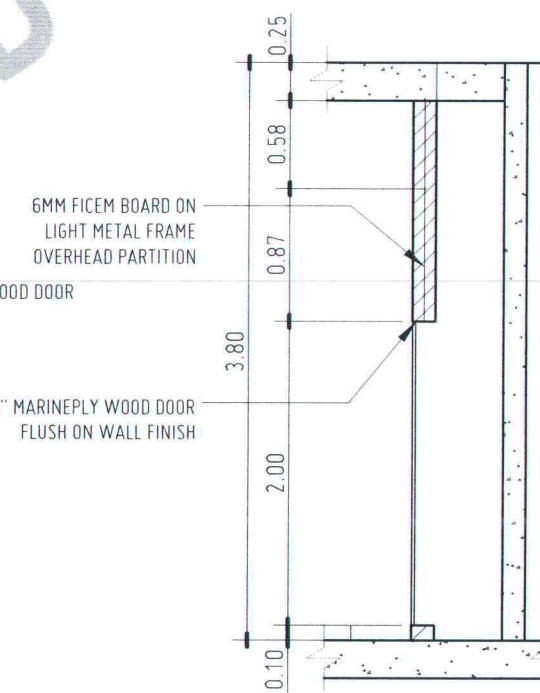
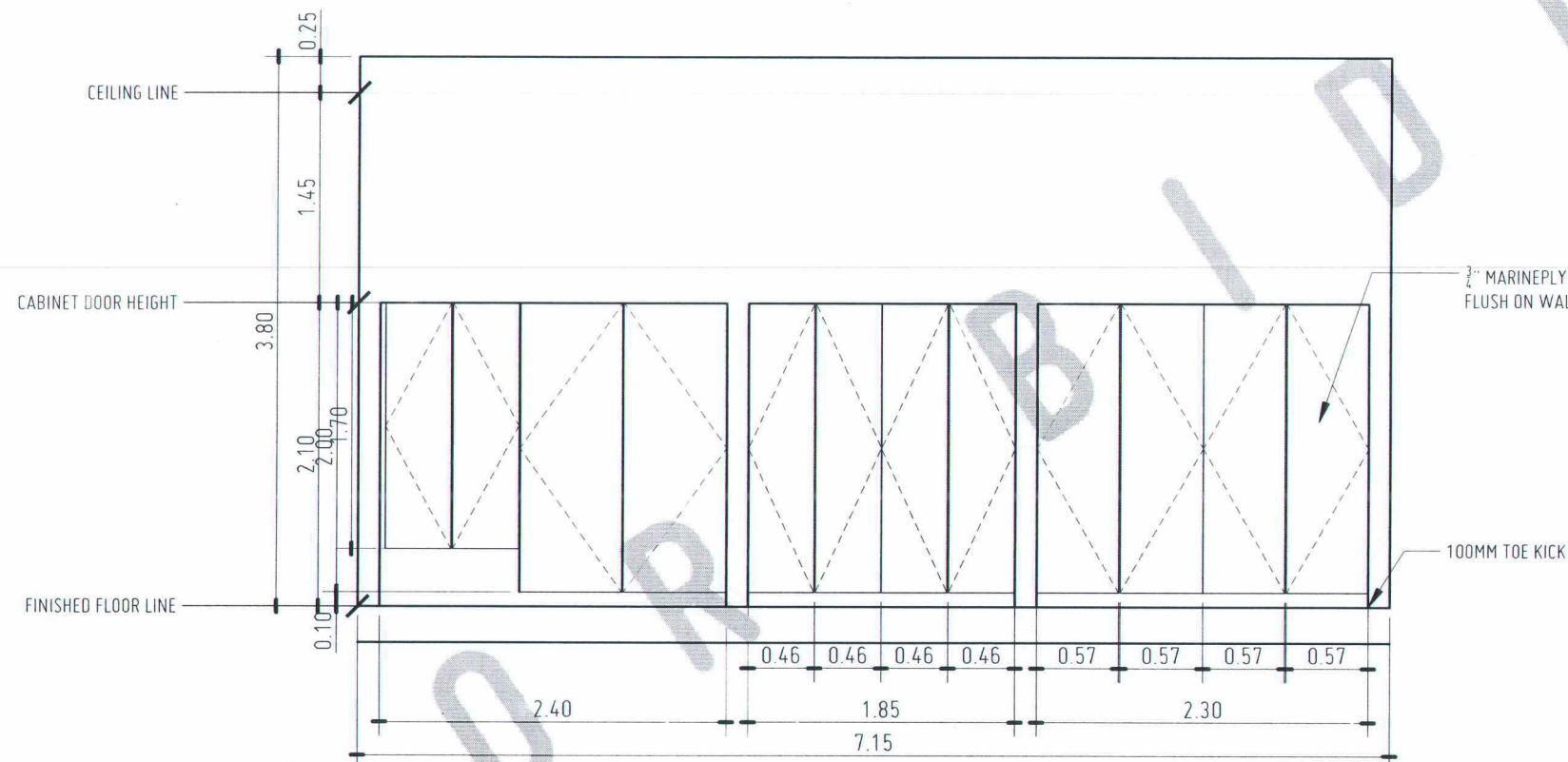
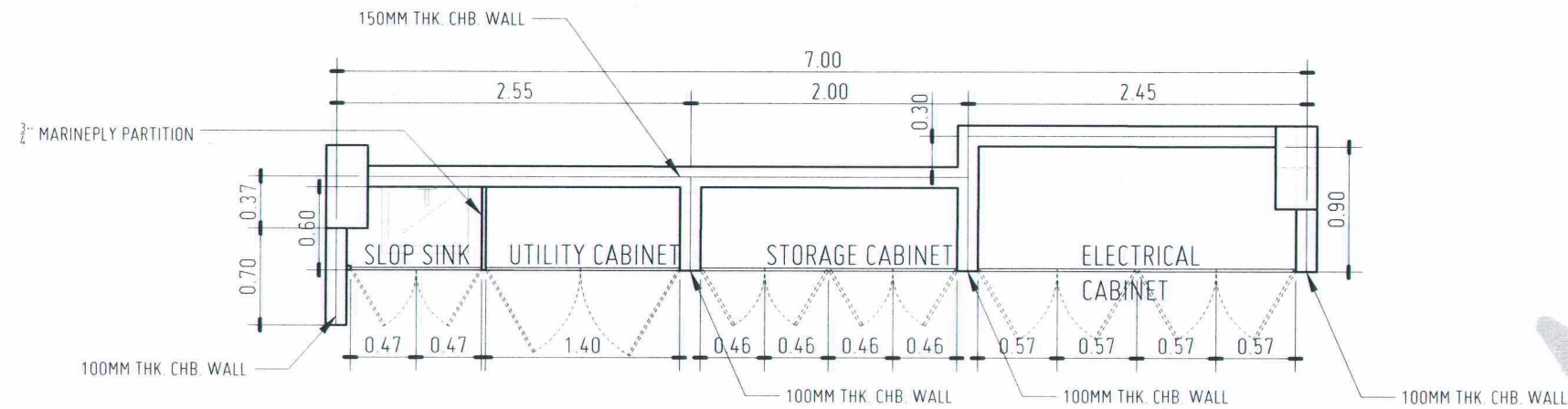
SECTION

TYPICAL STORAGE CABINET "A" DETAIL

SCALE:

1:50 MTS





ELEVATION

SECTION



TYPICAL STORAGE CABINET "B" DETAIL

SCALE:

1:50 MTS



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ARCHITECT
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DATE 02-10-2026
TIN NO. 185-092-837 PLACE CDO

PROJECT
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DR. AMBROSIO B. CULTURA II
PRESIDENT, USTP SYSTEM

SHEET CONTENTS:

DRAWN BY:
DATE DRAWN:
FNT:

A44

1. GENERAL

- THE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, OPENINGS AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORKS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK.
- THE STRUCTURAL DRAWINGS HEREIN REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN. SIMILAR DETAILS OR CONSTRUCTION SHALL BE USED, SUBJECT TO THE APPROVAL BY THE ROYAL COMMISSION.
- LOADING(S) APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADING USED IN THE DESIGN OF THIS STRUCTURE ARE INDICATED IN THE "DESIGN CRITERIA NOTES". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE.
- ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS OF THESE STANDARDS, UNLESS OTHERWISE NOTED.

2. DESIGN CRITERIA

2.1 STANDARDS

- THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING CODE OF STANDARD PRACTICE:

- NATIONAL STRUCTURAL CODE OF THE PHILIPPINES (NSCP, vol. 1, 2015 7th ed)
- ASSOCIATION OF STRUCTURAL ENGINEERS OF THE PHILIPPINES
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- STRUCTURAL ENGINEERING INSTITUTE/AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE 7-02, 2nd ed)

2.2 MATERIALS

- THE FOLLOWING STRUCTURAL MATERIALS AND THEIR CORRESPONDING STRENGTH ARE ADOPTED IN THE DESIGN:

A. CONCRETE CYLINDER STRENGTH AT 28 DAYS	$f'_c = 28 \text{ MPa}$
B. CONCRETE BLINDING	$f'_c = 20 \text{ MPa}$
C. REINFORCING BARS	$f_y = 414 \text{ MPa}$
D. CONCRETE MASONRY UNITS (LOAD BEARING)	$f_m = 5.175 \text{ MPa}$
E. CONCRETE MASONRY UNITS (NON-LOAD BEARING)	$f_m = 2.75 \text{ MPa}$
F. STRUCTURAL STEEL PLATES AND SHAPES - ASTM A36	$f_y = 248 \text{ MPa}$
G. ANCHOR BOLTS - ASTM A307	$f_y = 278 \text{ MPa}$

2.3 DESIGN LOADS

- DEAD LOADS. DESIGN GRAVITY DEAD LOADS USED IN THE DESIGN ARE AS FOLLOWS:

A. CONCRETE	24 KN/M ²
B. 50 MM FLOOR TOPPING - FINISH	1.7 KN/M ²
C. 100 MM CONCRETE MASONRY UNITS	7.40 KN/M ²
D. 150 MM CONCRETE MASONRY UNITS	7.80 KN/M ²
E. ROOF TOPPING - WATERPROOFING - INSULATION	2.50 KN/M ²

- LIVE LOADS. DESIGN GRAVITY LIVE LOADS USED IN THE DESIGN ARE AS FOLLOWS:

A. FLOOR LIVE LOAD	2.0 KN/M ²
B. ROOF LIVE LOAD	CONCRETE ROOF: 1.8 KN/M ² STEEL ROOF: 0.8 KN/M ²
C. CORRIDORS, STAIRS & BALCONIES	5.0 KN/M ²

2.3.3 WIND LOAD

- WIND LOAD ON STRUCTURE SHALL BE COMPUTED AND APPLIED IN CONFORMANCE TO THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, LATEST EDITION.

2.3.2 DESIGN WIND PRESSURES

- BASIC WIND VELOCITY FOR ALUBIJID, MISAMIS OR ARE AS FOLLOWS: 290 KPH
- SIMPLIFIED DESIGN WIND SHALL BE DESIGNED AS GIVEN IN NSCP FIGURE REFS. 8 & 13.

2.3.3 ADJUSTMENT FACTOR FOR BUILDING HEIGHT AND EXPOSURE (A)

- ADJUSTMENT FACTOR FOR BUILDING HEIGHT AND EXPOSURE (A) SHALL BE AS GIVEN IN FIGURE 5-2 OF SEI/ASCE 7.

2.3.4 IMPORTANCE FACTOR (I)

- IMPORTANCE (I) SHALL BE AS DEFINED IN SECTION 5.5.5 OF SEI/ASCE 7.

2.3.5 EXPOSURE CATEGORY

- AN EXPOSURE CATEGORY SHALL BE IN ACCORDANCE WITH SECTION 5.5.5 OF SEI/ASCE 7.

2.3.6 EARTHQUAKE LOADS

- COMPUTATIONS AND APPLICATION OF SEISMIC LOADS ON THE STRUCTURE SHALL BE IN ACCORDANCE WITH THE EARTHQUAKE PROVISIONS OF THE INTERNATIONAL BUILDING CODE, LATEST EDITION.

3. CONCRETE

3.1. GENERAL

- STRUCTURAL CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
- CONCRETE SHALL BE READY MIXED UNLESS OTHERWISE APPROVED.
- CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI STANDARD.
- REINFORCEMENT FOR CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF ACI BUILDING CODE 318/318R-25.

3.2 FOUNDATION

- FOUNDATION SHALL BE DESIGNED BASED ON THE GEO-TECHNICAL INVESTIGATION REPORT. THE GENERAL CONTRACTOR SHALL CONDUCT HIS OWN GEO-TECHNICAL TEST TO VERIFY THE DESIGN IS SAFE AND IS ACCORDANCE TO THE GEO-TECHNICAL SOIL TEST RESULT REPORT.

3.2.2 SITE PREPARATION

- ALL VEGETATION, DEBRIS AND SURFACE SOILS CONTAINING ORGANIC MATERIALS SHALL BE REMOVED FROM THE SITE.
- THE EXPOSED SUB-GRADE SHALL BE DENSIFIED PRIOR TO PLACING ANY FILL. THE DENSIFICATION SHALL BE ACHIEVED BY COMPACTION, AND SHALL CONTINUE UNTIL THE SOIL DENSITY, 300 MM BELOW THE EXISTING SUB-GRADE, IS AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY.
- IF THE COMPACTED SOILS ARE DUE TO CONSTRUCTION ACTIVITIES, THESE SOILS SHALL BE RE-COMPACTED AND RE-TESTED.

3.2.3 GROUND WATER TABLE

- THE CONTRACTOR'S ATTENTION IS DRAWN TO GROUNDWATER ELEVATIONS INDICATED OR DISCUSSED IN THE GEO-TECHNICAL REPORT. WELL-POINTS MAY BE NEEDED SO THAT THE GROUND WATER LEVEL IS MAINTAINED AT LEAST 500 MM BELOW THE BOTTOM OF FOOTINGS DURING COMPACTION AND CONSTRUCTION.

- THE SIDES OF FOOTINGS SHALL BE FORMED, EXCAVATED SOILS, TO FORM THE SIDES OF FOOTINGS ARE NOT ACCEPTABLE.

- TOP OF FOOTING ELEVATION SHALL BE AS SHOWN ON THE FOUNDATION PLAN OR DETAILS. THESE ELEVATIONS ARE A MAXIMUM AND SHALL BE LOWERED AS REQUIRED TO OBTAIN THE REQUIRED DESIGN BEARING PRESSURE.

- NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST FOUNDATION WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY BRACING PERMANENT CONSTRUCTION.

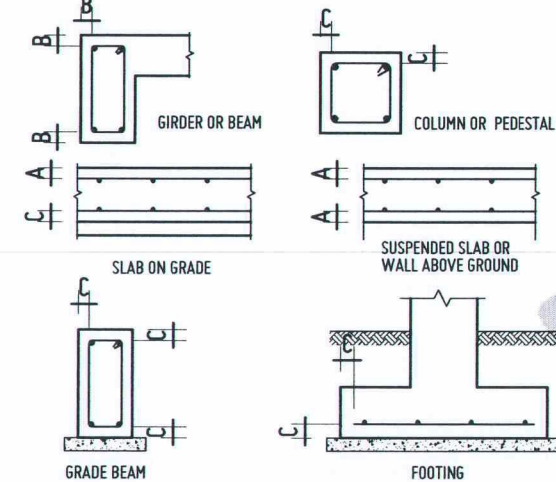
4. CONCRETE CONSTRUCTION

- NO SPLICES SHALL BE PERMITTED IN FLEXURAL MEMBER REINFORCEMENT AT POINTS WHERE CRITICAL BENDING STRESSES OCCURS.

- WHEN A BEAM CROSSES A GIRDER, REST BEAM BARS ON TOP OF GIRDER BARS.

- IF SLABS ARE REINFORCED BOTH WAYS (TWO-WAY SPANNING SLABS), THE BARS ALONG THE SHORT SPAN SHALL BE AT THE LOWER LAYER FOR BOTTOM BARS, AND UPPER LAYER FOR TOP BARS.

- UNLESS OTHERWISE NOTED, THE FOLLOWING CLEAR CONCRETE COVER OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 308-14, CLEAR COVER REQUIREMENTS AND USED AS FOLLOWS:

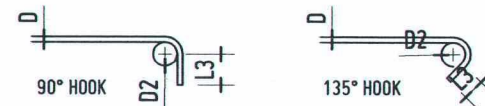


CAST-IN-PLACE CONCRETE			
CLEAR COVER	EXPOSED TO WEATHER OR EARTH	NOT EXPOSED TO WEATHER OR EARTH	
A	50	25	
B	50	40	
C	75	50	
PRECAST CONCRETE			
CLEAR COVER	EXPOSED TO EARTH	EXPOSED TO WEATHER	NOT EXPOSED TO WEATHER OR EARTH
WALL PANELS	75	30	20
OTHERS	75	50	30

- HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE FOLLOWING MINIMUM REQUIREMENT:

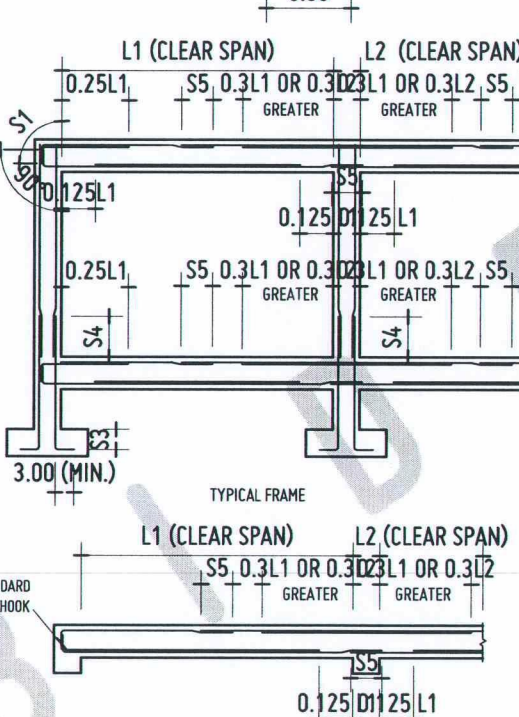
MAIN REINFORCEMENT			
BARS (MM)	MINIMUM BEND DIAMETER	EXTENSION	
	D1 (MM)	L1 (MM)	L2 (MM)
#8	50	60	95
#10	60	60	125
#12	70	60	145
#14	85	60	170
#16	95	65	190
#18	110	70	215
#20	130	80	245
#22	150	90	275
#25	175	100	300
#28	225	110	335

GENERAL NOTES



STIRRUPS AND TIES		
BARS (MM)	MINIMUM BEND DIAMETER	EXTENSION
	D2 (MM)	L3 (MM)
#8	30	50
#10	40	60
#12	50	70
#14	55	85
#16	65	95

- UNLESS OTHERWISE INDICATED, THE FOLLOWING MINIMUM DEVELOPMENT AND LAP LENGTH SHALL BE USED:



BARS (MM)	MIN. DEVELOPMENT LENGTH			MIN. LAP LENGTH		
	IN TENSION		IN COMPRESSION	IN TENSION		IN COMPRESSION
	S1 (MM)	S2 (MM)	S3 (MM)	S4 (MM)	S5 (MM)	
#8	420	325	200	420	300	
#10	525	405	210	525	300	
#12	630	485	250	630	355	
#14	735	565	295	735	410	
#16	840	645	335	840	470	
#18	945	725	380	945	530	
#20	1050	805	420	1050	590	
#22	1140	885	460	1140	650	
#25	1260	965	505	1260	715	
#28	1380	1045	550	1380	780	
#30	1500	1125	600	1500	840	
#32	1620	1205	650	1620	900	

NOTES

- THE INDICATED MINIMUM DEVELOPMENT AND LAP LENGTH IS FOR CLASS 25 CONCRETE. FOR OTHER THAN 25 MPa, THE VALUES OF S1, S2, S3, AND S4 SHALL BE MULTIPLIED BY $\sqrt{f'_c/28}$.
- WHEN LIGHTWEIGHT AGGREGATE CONCRETE IS USED, INDICATED VALUE OF S1 AND S2 SHALL BE MULTIPLIED BY 1.3. HOWEVER WHEN THE AVERAGE SPLITTING TENSILE STRENGTH OF LIGHTWEIGHT AGGREGATE CONCRETE (f_{ct}) IS SPECIFIED, S1 AND S2 SHALL BE PERMITTED TO BE MULTIPLIED BY $\sqrt{f_{ct}/1.8f_{ct}}$ BUT NOT LESS THAN 1.0.
- FOR EPOXY COATED BARS, S1 AND S2 SHALL BE MULTIPLIED BY 1.3.
- LAP AND DEVELOPMENT LENGTH OF INDIVIDUAL BARS WITHIN A BUNDLE SHALL BE THAT FOR INDIVIDUAL BAR INCREASED BY 70% FOR THREE BAR BUNDLE, 33% FOR FOUR BAR BUNDLE.

5. MASONRY

- MASONRY UNITS SHALL CONFORM TO THE REQUIREMENTS OF ACI 530/ASCE 5.
- MASONRY EXPANSION AND CONTROL JOINT REQUIREMENTS SHALL BE AS INDICATED.
- UNLESS OTHERWISE INDICATED, ALL WALLS SHALL BE LAID IN RUNNING BOND.
- REINFORCING BARS SHALL BE PROVIDED OF THE GIVEN SIZE AND SPACING AS INDICATED. PROVIDE BARS AT ALL WALL CORNERS, INTERSECTIONS AND OPENING EDGES.
- REBAR DOWELS SHALL BE PROVIDED FROM FOUNDATIONS TO MATCH VERTICAL BARS.
- LINTEL BEAM SHALL BE PROVIDED ABOVE ALL WALL OPENINGS.

6. STEEL

- ALL STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF AISC 360-15.

7. NOTES FOR DRAWING NOMENCLATURES

- ALL DIMENSIONS ARE IN MILLIMETERS AND ELEVATIONS ARE IN METERS UNLESS OTHERWISE NOTED.
- INDICATED DIMENSION SHALL PREVAIL AGAINST SCALED DIMENSION.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS, SHALL NOTIFY THE PROJECT OF ANY DISCREPANCY.
- ALL REINFORCING BARS SHALL BE READ AS FOLLOWS:



8. ITEM 508(1) HAND- LAID ROCK EMBANKMENT

- Hand-Laid Rock Embankment consists of hand-laid rock embankment, as designated in the Bid Schedule, constructed in accordance with this Specification and in conformity with the lines and grades shown on the Plans or established by the Engineer.

- Stones shall be sound and durable and furnished in a well-balanced range of sizes meeting the requirements herein. Unless otherwise provided by the Plans or Special provisions, all stones shall be more than 0.015 cubic meter in volume and not less than 75 percent of the total volume of rock embankment and shall consist of stones 0.03 cubic meter in volume. Stone obtained from excavation on performed under this contract may be used. Adobe stone shall not be used, unless otherwise specified.

- Sufficient excavation shall be made to expose a foundation bed that is satisfactory to the Engineer. The stones shall be founded on this bed and laid to the lines and dimensions required.

- Stones shall be laid flat and securely placed with broken joint lines. The larger stones shall generally be located in the lower part of the structure and voids shall be eliminated to the extent possible. Spalls smaller than the minimum stone size specified in Section 508.2, Material Requirements, shall be used to check the larger stones solidly in position and to substantially fill voids between the major stones as laid in the embankment. The exposed face of the rock mass shall be reasonably uniform, with no projection of more than 150 mm, beyond the neat lines shown on the Plans or as directed by the Engineer. Backfill adjacent to the hand-laid rock embankment shall be filled entirely with acceptable material coming from excavation items and compacted.

8. ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AP	ALTERNATELY PLACED
AR	ALTERNATELY REVERSED
AS	ALTERNATELY STAGGERED
ASTM	AMERICAN SOCIETY FOR TESTING
AND MATERIALS	
AWS	AMERICAN WELDING SOCIETY
B	BOTTOM
BF	BACK FACE
BW	BOTHWAYS
CL	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CONT	CONTINUOUS
DIA	DIAMETER
DISCONT	DISCONTINUOUS
DTL	DETAIL
DWG	DRAWING
EF	EXISTING FACE
EL	EXISTING LEVEL
ELEV	ELEVATION
EQ	EQUAL
FW	EACH WAY
EXT	EXTERIOR
FF	FRONT FACE
FIN	FINISH
GLS	GUIDELINE SPECIFICATION
HOR	HORIZONTAL
INT	INTERIOR
LGTH	LENGTH
MIN	MINIMUM
NIS	NOT TO SCALE
ON CENTER	ON CENTER
REBAR	REINFORCING BAR
REINF	REINFORCED, REINFORCEMENT OR
REINFORCING	
SEC	SECTION
SP	SPACING
STIRR	STIRRUPS
SYMM	SYMMETRICAL
T	TOP
THK	THICK
TYP	TYPICAL
UDN	UNLESS OTHERWISE NOTED
VERT	VERTICAL
WWF	WELDED WIRE FABRIC

9. WATER PROOFING

- ALL STRUCTURAL CONCRETE IN CONTACT WITH THE GROUND SHALL BE PROTECTED EITHER BY TWO COATS OF BITUMINOUS DAMPROOFING OR BY USE OF WATER PROOF MEMBRANE AS BEEN SHOWN ON THE DETAILED DRAWING.

- POLYETHYLENE SHEET OF 200 MICRON TO BE USED AS VAPOR BARRIER UNDERMUD SLAB.

- WATER PROOFING TO BE APPLIED ON TOP OF MUD CONCRETE AND ABOVE.

- WHEN USING WATER PROOF MEMBRANE, IT SHOULD BE AS PER RC GUIDE SPEC. 07110 (TANKING SYSTEM) ROYAL COMMISSION GUIDELINE SPECIFICATION 07130.



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PRC NO. 0013329 RFR NO. 1014386A
DATE 01-23-2025
TIN NO. 199-292-007 PLACE EL SALVADOR CITY

Construction of Innovation and Collaboration
Hub Phase II, Alubijid (Main) Campus
USTP AGILA S&T PARK, ALUBIJID, MISAMIS ORIENTAL
OWNER: UNIVERSITY OF SCIENCE & TECHNOLOGY OF SOUTHERN PHILIPPINES

RECOMMENDING APPROVAL:
ENGR. GRACE C. BABA
DIRECTOR, INFRASTRUCTURE PLANNING & FACILITY DEVELOPMENT OFFICE

RECOMMENDING APPROVAL:
DR. LORY LIZA D. BULAY-OG
DIRECTOR, USTP ALUBIJID

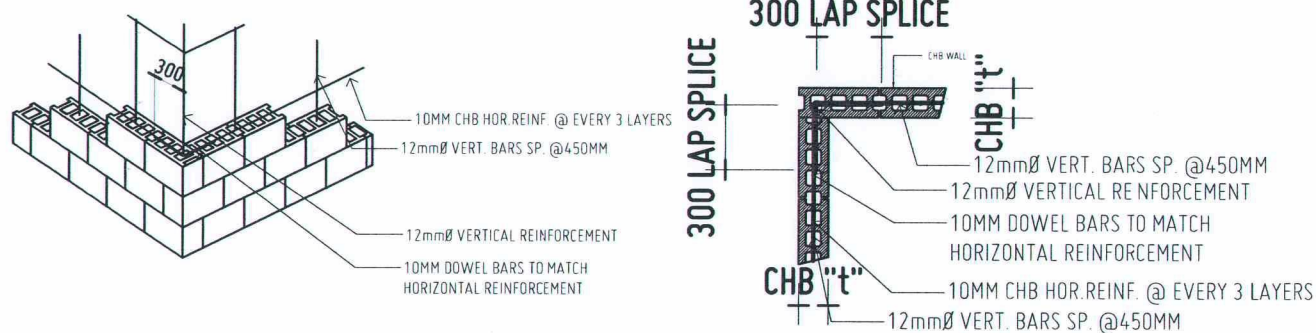
RECOMMENDING APPROVAL:
ATTY. JONATHAN S. OCHE
VP FOR ADMINISTRATION & LEGAL AFFAIRS

APPROVED BY:
DR. AMBROSIO B. CULTURA II
PRESIDENT, USTP SYSTEM

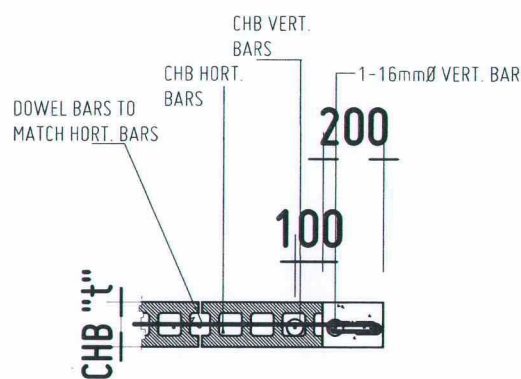
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DATE DRAWN:
FMT:

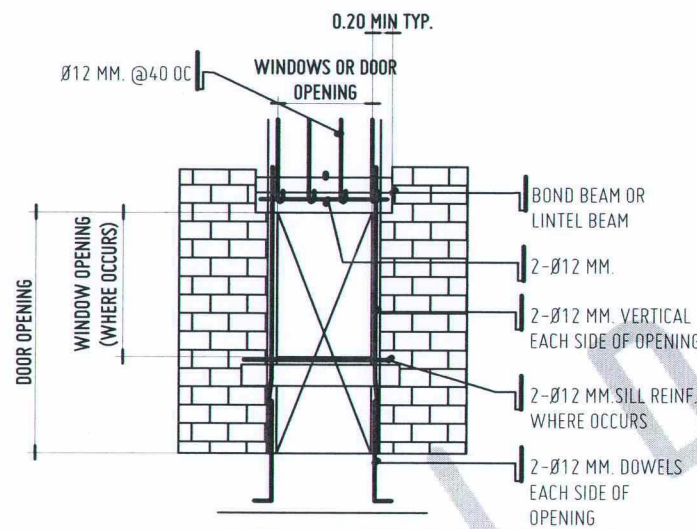
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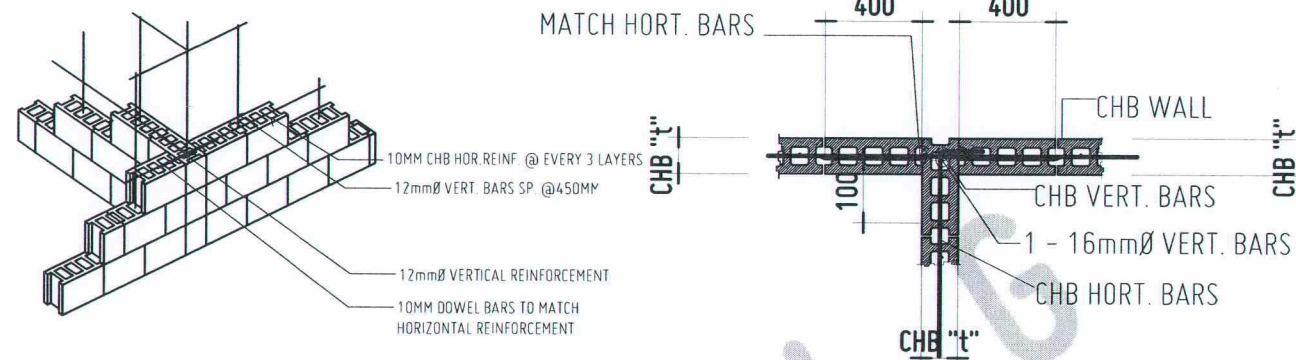
TYPICAL CONNECTION DETAIL OF
CORNER WALL
1:50



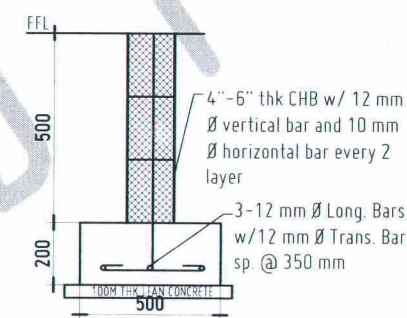
TYPICAL CONNECTION AT
END/OPENING OF MASONRY WALL
1:50



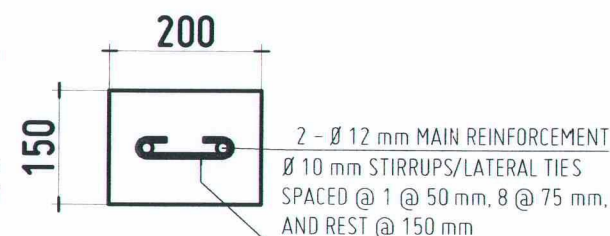
TYPICAL WALL OPENING DETAIL
1:100



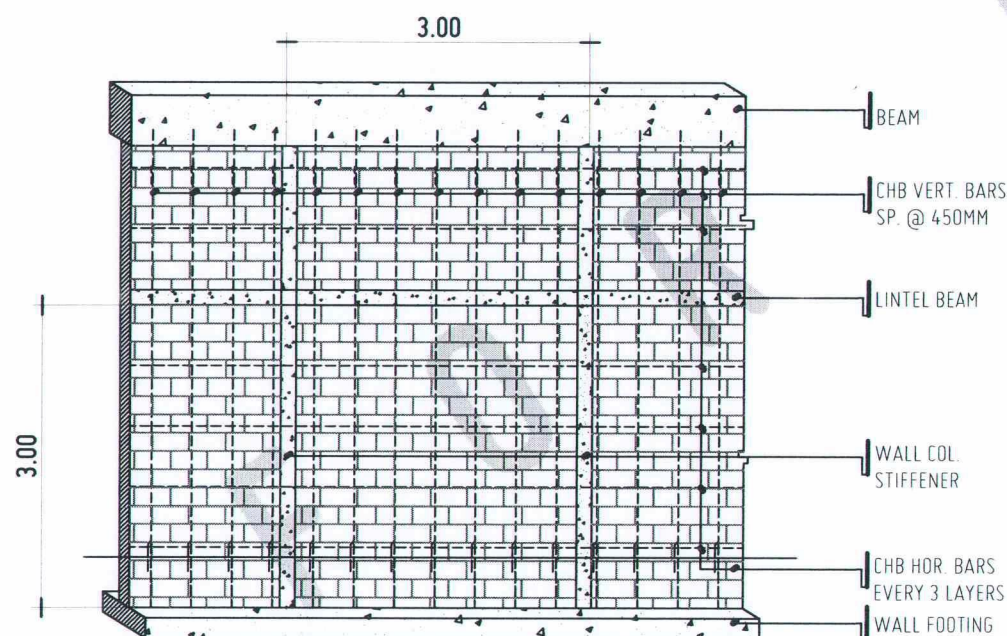
TYPICAL CONNECTION DETAIL OF
WALL INTERSECTION
1:50



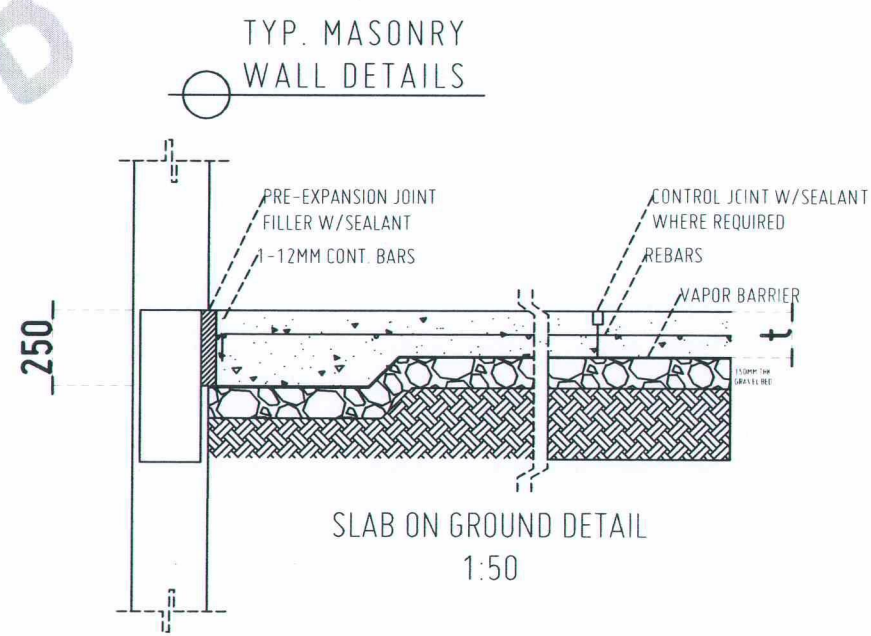
WALL FOOTING (WF)
DETAILS
SCALE: 1:20 MTS



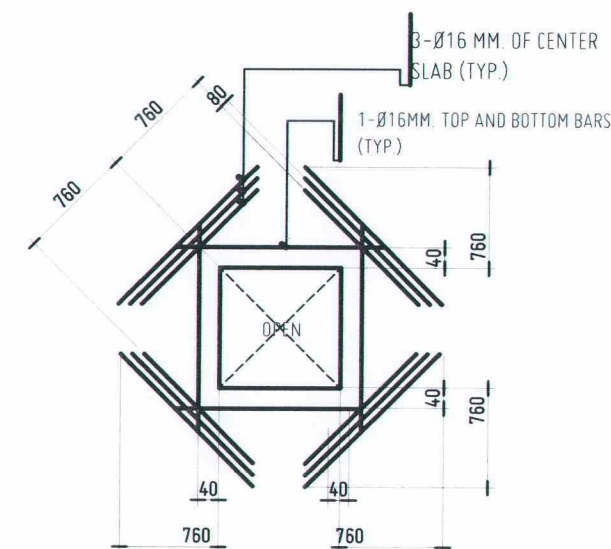
LINTEL AND STIFFENER
BEAM DETAILS
SCALE: 1:10 MTS



TYPICAL WALL CONSTRUCTION
1:75



TYP. MASONRY
WALL DETAILS
TYP. STRUCTURAL
DETAILS



TYPICAL SLAB OPENING DETAIL
1:50

Approved by:

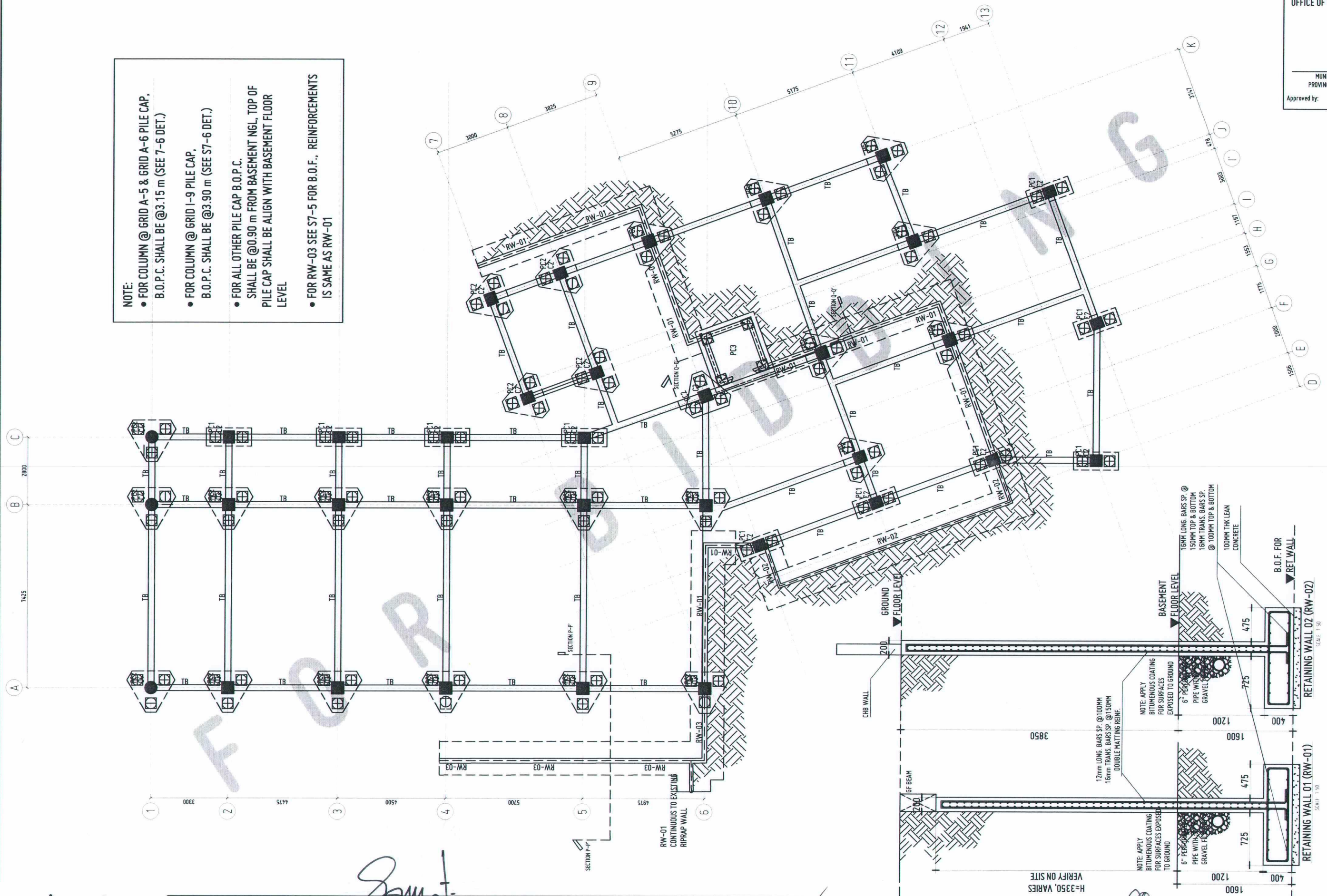
FOUNDATION LAYOUT PLAN

SCALE: 1:150 MTS

2013

CALL:

SCALE: 1:50 MTS



REPUBLIC OF THE PHILIPPINES
UNIVERSITY OF SCIENCE AND TECHNOLOGY OF SOUTHERN PHILIPPINES
CAGAYAN DE ORO CAMPUS
INFRASTRUCTURE PLANNING AND FACILITY DEVELOPMENT UNIT
CLARO M. RECTO AVENUE, LAPASAN, CAGAYAN DE ORO CITY 9000
TELEPHONE # (088) 2272-60-65 / (088) 956-1738 / 956-1739 | TELE FAX (088) 9356-4596
WEBSITE: www.ustp.edu.ph

ENGR. ERNESTO CH. QUIJOTE
CIVIL/STRUCTURAL ENGINEER

PRC NO.	0013329	PTR NO.	191438
		DATE	01-23-20
TIN NO.	185-092-837	PLACE	EL SALVADOR C

PROJECT	Construction of Innovation and Collaboration Hub Phase II, Alubijid (Main) Campus
LOCATION	USTP AGILA S&T PARK, ALUBIJID, MISAMIS ORIENTAL
OWNER	UNIVERSITY OF SCIENCE & TECHNOLOGY OF SOUTHERN PHILIPPINES

RECOMMENDING APPROVAL:



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DIRECTOR, INFRASTRUCTURE PLANNING
& FACILITY DEVELOPMENT OFFICE

RECOMMENDING APPROVAL:

Lory Liza D. Bulay

DR. LORY LIZA D. BULAY-OG

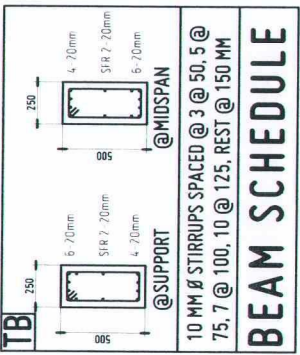
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